

# FINITE ELEMENT ANALYSIS OF RIGID PAVEMENT USING EVERFE2.24 & COMPARISON OF RESULTS WITH IRC58-2002 & IRC58-2015

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## ABSTRACT

*In this study analysis of plain cement concrete pavement was done with 3-D mechanistic FEM computer programme EVERFE2.24. This programme was developed by Bill David, University of Maine, USA. Rigid pavement is modelled as a flat slab with DLC as base course and subgrade beneath it.*

*Stresses in rigid pavement at critical location was calculated due to combined effect of axle load and environmental factor. These results are compared with IRC58-2015 & 2002. The disparity between results are analysed and plotted on graph.*

*This study finds that stresses given by IRC58-2015 is up to 42% less than that given by IRC58-2002, and stresses given by EverFE2.24 is nearly same as given by IRC58-2002. It also highlighted some issues related to new code of design i.e. IRC58-2015.*

## KEYWORDS

*FEM, EVERFE2.24, DLC, RIGID PAVEMENT.*

## INTRODUCTION

During the past decade the use of three dimensional (3-D) finite element (FE) method for analysing stresses in rigid pavement have grown significantly. The finite element method enables researchers to better understand the critical behaviour of pavement which cannot be captured with conventional analytical methods such as slab- base shear transfer, load transfer efficiency of dowel and tie bar and response of pavement due to dynamic loading.

In Mathematics, finite element method (FEM) is a numerical technique for finding approximate solution to boundary value problem for partial differential equation. It uses subdivision of a whole problem domain into simpler parts, called finite elements, and variational methods from the calculus of variation to solve the problem by minimizing an associated error function. Analogous to the idea that connecting many tiny straight lines can approximate a larger circle, FEM encompasses methods for connecting many simple element equations over many small subdomains, named finite elements, to approximate a more complex equation over a larger domain.

In this study rigid pavement is modelled as flat slab having dimension 4.5 x 3.5m which is generally taken in design. Subgrade is considered as tensionless dense liquid foundation and DLC as base course. Various material properties of component layer and loading pattern are varied to

observe the effect of these factors on pavement stresses. The main objectives of the study are following:

- To study the effect of modulus of sub grade reaction on pavement response using EverFE2.24.
- To study the effect of temperature differential on pavement stresses using EverFE2.24.
- To study the effect of slab thickness on pavement stresses using EverFE2.24.
- Compare the results given by EverFE2.24 with IRC58-2002&IRC58-2015.
- Reveal the disparity and showing it through graph.

## EVERFE2.24

EverFE2.24 is 3D finite element analysis programme for JPCC pavement. It was developed by Bill David, University of Maine, USA. It employs several element types to discretize rigid pavement system having from one to nine slab units. In this upto three elastic base layer below the slab can be specified and subgrade can be modelled as dense liquid foundation with tension less or tension supporting. Twenty-node quadratic hexahedral elements are used to discretize the slabs and elastic base layers and the dense liquid foundation is incorporated via numerically integrated, 8-noded quadratic elements that are meshed with the bottom-most layer of solid elements. Linear or nonlinear aggregate interlock joint load transfer as well as dowel load transfer can be modelled at transverse joints. Load transfer across longitudinal joints via transverse tie bars can also be modelled.

## DETAIL OF THE MODEL USED IN THE STUDY

1. Slab dimension = 4.5 x 3.5m
2. Slab Thicknesses = 20cm, 25cm and 30cm.
3. Modulus of elasticity of slab,  $E = 30000 \text{ Mpa}$
4. Poisson's ratio,  $\mu = 0.15$ , density,  $\gamma = 2400 \text{ kg/m}^3$
5. Coefficient of thermal expansion,  $\alpha = 10 \times 10^{-6} / ^\circ\text{C}$
6. Positive linear Temperature differential with  $\Delta t = 10^\circ\text{C}$  (10-0, 20-10, 30-20, 60-50) is taken.
7. Base is 100mm DLC (dry lean concrete) with  $E = 13600 \text{ Mpa}$ ,  $\mu = 0.20$  &  $\gamma = 2400 \text{ kg/m}^3$
8. There are three types of subgrade is taken with  $k$  value 42MPa/m, 48MPa/m & 55MPa/m. Since the base is 100mm DLC, So Effective  $k$  became 166MPa/m, 208MPa/m & 278MPa/m respectively.
9. Loading = 100KN (single axle dual wheel placed critically according to IRC58-2015 as shown in screen shot of EverFE2.24, Fig1)

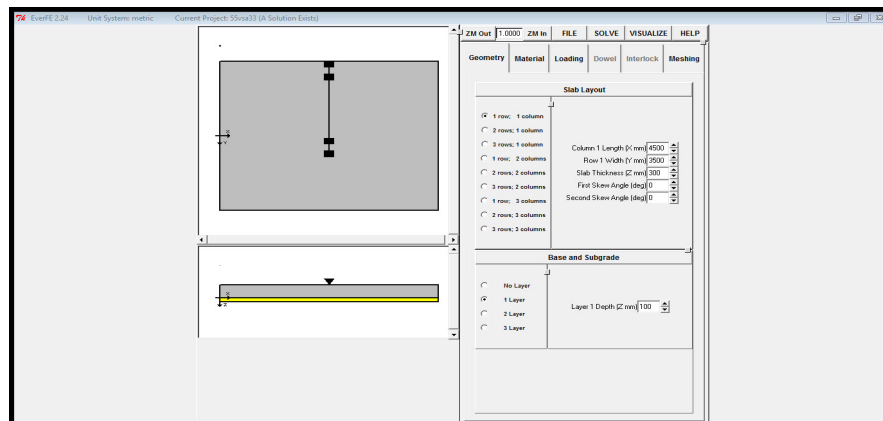


Fig1: Screen shot of EverFE2.24 to demonstrate loading pattern

## DETAILS OF RESULTS

The details of maximum edge stresses given by EverFE2.24, IRC58-2015 and IRC58-2002 for different type of pavement and loading condition are coated in table1.

Table 1: Value of maximum edge tensile stresses in concrete pavement.

k-value	Slab thickness(cm)	Temperature differential	Max tensile stresses(MPa)		
			IRC58-2002	IRC58-2015	EverFE2.24
42Mpa/m	20	10-0 <sup>0</sup> C	3.4853	2.8186	3.9393
		20-10 <sup>0</sup> C	3.4853	2.8186	3.9390
		30-20 <sup>0</sup> C	3.4853	2.8186	3.9387
		60-50 <sup>0</sup> C	3.4853	2.8186	3.9379
	25	10-0 <sup>0</sup> C	2.8052	1.9839	2.8722
		20-10 <sup>0</sup> C	2.8052	1.9839	2.8719
		30-20 <sup>0</sup> C	2.8052	1.9839	2.8716
		60-50 <sup>0</sup> C	2.8052	1.9839	2.8708
	30	10-0 <sup>0</sup> C	2.2709	1.5356	2.1156
		20-10 <sup>0</sup> C	2.2709	1.5356	2.1152
		30-20 <sup>0</sup> C	2.2709	1.5356	2.1169
		60-50 <sup>0</sup> C	2.2709	1.5356	2.1161
48Mpa/m	20	10-0 <sup>0</sup> C	3.4455	2.8066	3.9252
		20-10 <sup>0</sup> C	3.4455	2.8066	3.9249
		30-20 <sup>0</sup> C	3.4455	2.8066	3.9246
		60-50 <sup>0</sup> C	3.4455	2.8066	3.9166
	25	10-0 <sup>0</sup> C	2.8201	1.9707	2.8901
		20-10 <sup>0</sup> C	2.8201	1.9707	2.8898
		30-20 <sup>0</sup> C	2.8201	1.9707	2.8895
		60-50 <sup>0</sup> C	2.8201	1.9707	2.8888
	30	10-0 <sup>0</sup> C	2.3389	1.5209	2.1458
		20-10 <sup>0</sup> C	2.3389	1.5209	2.1456
		30-20 <sup>0</sup> C	2.3389	1.5209	2.1453
		60-50 <sup>0</sup> C	2.3389	1.5209	2.1466
55Mpa/m	20	10-0 <sup>0</sup> C	3.6903	2.7939	3.9086
		20-10 <sup>0</sup> C	3.6903	2.7939	3.9084
		30-20 <sup>0</sup> C	3.6903	2.7939	3.9005
		60-50 <sup>0</sup> C	3.6903	2.7939	3.8998
	25	10-0 <sup>0</sup> C	2.8189	1.9550	2.9075
		20-10 <sup>0</sup> C	2.8189	1.9550	2.9073
		30-20 <sup>0</sup> C	2.8189	1.9550	2.9070
		60-50 <sup>0</sup> C	2.8189	1.9550	2.9062
	30	10-0 <sup>0</sup> C	2.3831	1.5038	2.1775
		20-10 <sup>0</sup> C	2.3831	1.5038	2.1773
		30-20 <sup>0</sup> C	2.3831	1.5038	2.1770
		60-50 <sup>0</sup> C	2.3831	1.5038	2.1784

**PRESENTATION OF RESULTS**

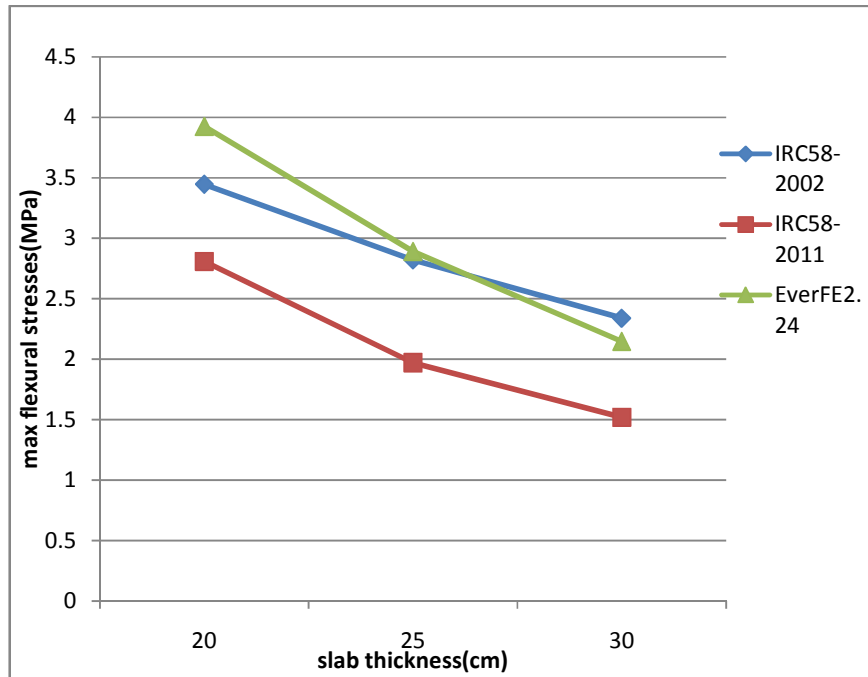


Fig2: Variation in maximum Stresses for  $\Delta t = 10^{\circ}\text{C}$  (30-20  $^{\circ}\text{C}$ ) and 100Kn (single axle dual wheel) in the concrete pavement for different thicknesses.

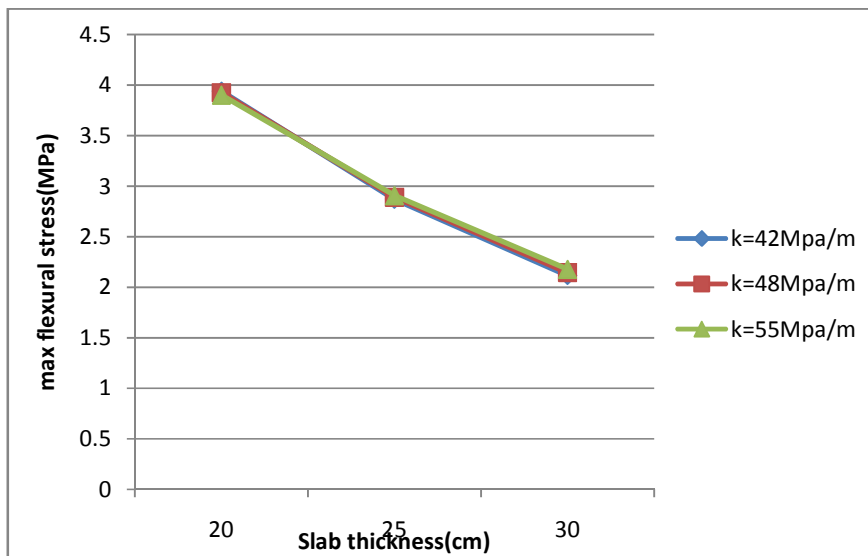


Fig3:Stresses in concrete pavement for different thicknesses given by EverFE2.24 due to  $\Delta t = 10^{\circ}\text{C}$  (30-20  $^{\circ}\text{C}$ ) and 100Kn (single axle dual wheel).

## DISCUSSION

### The main points of discussion are mentioned below:

1. In IRC58-2015 for stress calculation it uses effective k-value of foundation which is theoretically estimated for different combination of base as mentioned in table 3&4. If there is more than one layer is used between slab and subgrade then we cannot estimate the effective k value from these tables. In EverFE2.24 we use k-value of subgrade and define material properties such as E,  $\mu$  etc. of other upper layer below slab for estimating the foundation strength.

2. From fig2 we can see that stresses given by regression equation mentioned in appendix-5 of IRC58-2015 are considerably low from stresses calculated by EverFE2.24 and IRC 58-2002. This disparity is very high so cannot be considered as acceptable.

3. It is not clearly mentioned in IRC58-2015 for which dimension of slab the regression equation of appendix-5 is valid. Are these equations valid for 4.5x3.5m slab as used in illustrative problem then we should adopt more acceptable equation.

4. As flexural stresses are very much affected by wheel configuration, geometry of contact area of wheel, spacing between wheels of same axle etc. but these parameters are not considered in regression equation mentioned in appendix -5 of IRC58-2015.

5. In article 5.7.5 of IRC58-2015, it is mentioned that a polythene sheet is used between DLC and slab to reduce interlayer friction to allow relative movement between slab and DLC layer but in article 6.7.1 it is mentioned that by eliminating polythene sheet, the monolithic action of two layers can be exploited to reduce the pavement thickness, which is contradictory.

6. We observe that pavement stresses are slightly affected by range of temperature differential between upper layer and lower layer of slab as given by EverFE2.24. IRC58-2015 code uses only temperature differential. Although variation is very less here but for doweled and tied concrete pavement variation is more.

## CONCLUSION

The flexural stresses given by IRC58-2015 is upto 42% less than that given by IRC58-2002, which is very large disparity and stresses given by EverFE2.24 is nearly same as given by IRC58-2002. As various other anomalies are discussed above should be addressed so that we can make our code i.e. IRC 58-2015 more rational and acceptable.

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